

Appendix C
Structural Design Calculations

Port of Anchorage

Intermodal Expansion Project – 15% Concept Design

Preliminary Structural Design Calculations

February 2013

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US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 1 - Section B Trestle - 48" Hybrid Pile

By: **G. Jankhah**
 Date: **January 29, 2013**
 Checked By: **Vu Phan**
 Date: **February 11, 2013**

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 1 - Section B Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 14 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 182 |
| Ground Elevation (ft) | -27.4 | -23.4 | -19.4 | -15.4 | -11.4 | -7.4 | -3.4 | 0.6 | 4.6 | 7.4 |
| Top of BCF Elevation (ft) | -27.4 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 |
| Top of Silt/Mud (ft) | -27.4 | -23.4 | -19.4 | -15.4 | -11.4 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 2.6 | 6.6 | 10.6 | 14.6 | 17.4 |
| Thickness of Soft Mud (ft) | 0.0 | 1.6 | 5.6 | 9.6 | 13.6 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Pile Tip Elevation (ft) | -165.0 | -160.0 | -155.0 | -150.0 | -145.0 | -140.0 | -140.0 | -140.0 | -125.0 | -110.0 |
| Total Pile Length (ft) | 198.0 | 193.5 | 188.5 | 183.5 | 178.5 | 173.5 | 173.5 | 173.5 | 158.5 | 143.5 |
| Pile Length Above Ground Surface (ft) | 60.4 | 56.9 | 52.9 | 48.9 | 44.9 | 40.9 | 36.9 | 32.9 | 28.9 | 26.1 |
| Pile Embedment in the Soil (ft) | 137.6 | 136.6 | 135.6 | 134.6 | 133.6 | 132.6 | 136.6 | 140.6 | 129.6 | 117.4 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.75 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 | 45.0 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 25.5 | 15.0 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 72.9 |
| Crane Rail Beam | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 219.8 | 211.2 | 201.4 | 191.6 | 181.8 | 172.0 | 164.7 | 157.5 | 142.6 | 129.8 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 | 200.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | |
|---|--------|--------|--------|--------|--------|------|--------|--------|--------|------|
| Total Axial Demand (kips per pile) | 1259 | 1217 | 1205 | 1194 | 1182 | 1170 | 1162 | 1153 | 1017 | 705 |
| Nominal Axial Capacity (kips per pile) | 4831.8 | 4462.7 | 4093.5 | 3724.1 | 3354.6 | 2985 | 4462.7 | 3058.4 | 2641.2 | 2220 |
| Ultimate Axial Capacity (kips per pile) | 1957 | 1807 | 1658 | 1508 | 1359 | 1209 | 1807 | 1239 | 1070 | 749 |
| D/C Ratio | 0.64 | 0.67 | 0.73 | 0.79 | 0.87 | 0.97 | 0.64 | 0.93 | 0.95 | 0.94 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 1 - Section B BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
Date: January 29, 2013
Checked By: Vu Phan
Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 36-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 1 - Section B BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -39.9 | -36.7 | -33.5 | -30.3 |
| Top of BCF Elevation (ft) | -39.9 | -36.7 | -33.5 | -30.3 |
| Top of Silt/Mud (ft) | -39.9 | -36.7 | -33.5 | -30.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Pile Tip Elevation (ft) | -145.0 | -145.0 | -140.0 | -135.0 |
| Total Pile Length (ft) | 178.0 | 178.5 | 173.5 | 168.5 |
| Pile Length Above Ground Surface (ft) | 72.9 | 70.2 | 67.0 | 63.8 |
| Pile Embedment in the Soil (ft) | 105.1 | 108.3 | 106.5 | 104.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 232.1 | 227.5 | 219.1 | 210.8 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 kif, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|--------|--------|------|------|
| Total Axial Demand (kips per pile) | 1046 | 1067 | 1057 | 983 |
| Nominal Axial Capacity (kips per pile) | 3402.1 | 4044.2 | 3769 | 3035 |
| Ultimate Axial Capacity (kips per pile) | 1225 | 1456 | 1357 | 1093 |
| D/C Ratio | 0.85 | 0.73 | 0.78 | 0.90 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 1A - Section A Trestle - 48" Hybrid Pile

By: G. Jankhah
Date: January 29, 2013
Checked By: Vu Phan
Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 36-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 1A - Section A Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 14 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 182 |
| Ground Elevation (ft) | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 | 10.6 | 14.6 | 17.4 |
| Top of BCF Elevation (ft) | -17.4 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 |
| Top of Silt/Mud (ft) | -17.4 | -13.4 | -9.4 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 4.6 | 8.6 | 12.6 | 16.6 | 20.6 | 24.6 | 27.4 |
| Thickness of Soft Mud (ft) | 0.0 | 1.6 | 5.6 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Pile Tip Elevation (ft) | -155.0 | -150.0 | -145.0 | -140.0 | -135.0 | -130.0 | -125.0 | -125.0 | -115.0 | -100.0 |
| Total Pile Length (ft) | 188.0 | 183.5 | 178.5 | 173.5 | 168.5 | 163.5 | 158.5 | 158.5 | 148.5 | 133.5 |
| Pile Length Above Ground Surface (ft) | 50.4 | 46.9 | 42.9 | 38.9 | 34.9 | 30.9 | 26.9 | 22.9 | 18.9 | 16.1 |
| Pile Embedment in the Soil (ft) | 137.6 | 136.6 | 135.6 | 134.6 | 133.6 | 132.6 | 131.6 | 135.6 | 129.6 | 117.4 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.75 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 | 45.0 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 25.5 | 15.0 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 72.9 |
| Crane Rail Beam | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 196.6 | 188.0 | 178.2 | 168.4 | 158.6 | 148.8 | 139.0 | 131.8 | 119.4 | 106.6 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 | 200.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | |
|---|----------|----------|----------|----------|----------|------|------|--------|--------|------|
| Total Axial Demand (kips per pile) | 1231 | 1189 | 1178 | 1166 | 1154 | 1142 | 1131 | 1122 | 989 | 677 |
| Nominal Axial Capacity (kips per pile) | 3172.612 | 3134.714 | 3096.816 | 3058.919 | 3021.021 | 2985 | 2946 | 2865.1 | 2641.2 | 2220 |
| Ultimate Axial Capacity (kips per pile) | 1285 | 1270 | 1254 | 1239 | 1224 | 1209 | 1193 | 1160 | 1070 | 749 |
| D/C Ratio | 0.96 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.95 | 0.97 | 0.93 | 0.90 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 1 - Section A BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
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Purpose of Calculations:

To evaluate the axial capacity of 36-in hybrid pipe piles under the design loading and verify the pile embedments.

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4. Structural Design Criteria
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Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|--------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft^2 | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch^2 | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch^2 | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 1 - Section A BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|---------------|
| | 1 | 2 | 3 | 4 |
| Row | | | | |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -29.9 | -26.7 | -23.5 | -20.3 |
| Top of BCF Elevation (ft) | -29.9 | -26.7 | -23.5 | -20.3 |
| Top of Silt/Mud (ft) | -29.9 | -26.7 | -23.5 | -20.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Pile Tip Elevation (ft) | -145.0 | -145.0 | -140.0 | -135.0 |
| Total Pile Length (ft) | 178.0 | 178.5 | 173.5 | 168.5 |
| Pile Length Above Ground Surface (ft) | 62.9 | 60.2 | 57.0 | 53.8 |
| Pile Embedment in the Soil (ft) | 115.1 | 118.3 | 116.5 | 114.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 214.0 | 209.4 | 201.1 | 192.7 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|---------|---------|---------|---------|
| Total Axial Demand (kips per pile) | 1024 | 1045 | 1035 | 962 |
| Nominal Axial Capacity (kips per pile) | 3405.33 | 4052.81 | 3773.79 | 3036.97 |
| Ultimate Axial Capacity (kips per pile) | 1226 | 1459 | 1359 | 1093 |
| D/C Ratio | 0.84 | 0.72 | 0.76 | 0.88 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 1 - Section C Access Trestle - 48" Hybrid Pile

By: **G. Jankhah**
 Date: **January 29, 2013**
 Checked By: **Vu Phan**
 Date: **February 11, 2013**

Purpose of Calculations:

To evaluate the axial capacity of 36-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 3 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 1 - Section C Access Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 188 | 208 | 228 |
| Ground Elevation (ft) | -37.4 | -33.4 | -29.4 | -25.4 | -21.4 | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 |
| Top of BCF Elevation (ft) | -45.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 |
| Top of Silt/Mud (ft) | -45.0 | -41.0 | -41.0 | -36.0 | -31.0 | -26.0 | -21.0 | -19.0 | -19.0 | -19.0 | -19.0 | -19.0 |
| Thickness of Backfill (ft) | 7.6 | 7.6 | 11.6 | 10.6 | 9.6 | 8.6 | 7.6 | 9.6 | 13.6 | 17.6 | 21.6 | 25.6 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 5.0 | 10.0 | 15.0 | 20.0 | 22.0 | 22.0 | 22.0 | 22.0 | 22.0 |
| Pile Tip Elevation (ft) | -165.0 | -160.0 | -155.0 | -150.0 | -145.0 | -145.0 | -140.0 | -135.0 | -130.0 | -145.0 | -140.0 | -125.0 |
| Total Pile Length (ft) | 198.0 | 193.5 | 188.5 | 183.5 | 178.5 | 178.5 | 173.5 | 168.5 | 163.5 | 178.5 | 173.5 | 158.5 |
| Pile Length Above Ground Surface (ft) | 70.4 | 66.9 | 62.9 | 58.9 | 54.9 | 50.9 | 46.9 | 42.9 | 38.9 | 34.9 | 30.9 | 26.9 |
| Pile Embedment in the Soil (ft) | 127.6 | 126.6 | 125.6 | 124.6 | 123.6 | 127.6 | 126.6 | 125.6 | 124.6 | 143.6 | 142.6 | 131.6 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |

Note

Based on 5:1 Slope

Estimated, based on North Extension As-built Drawing

Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 |
| 3" Asphalt Overlay | 13.5 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 12.8 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 83.7 |
| Crane Rail Beam | | | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 237.9 | 229.2 | 219.4 | 209.6 | 199.8 | 192.6 | 182.8 | 173.0 | 163.2 | 163.7 | 153.9 | 139.0 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | | | |
|---|--------|------|--------|--------|--------|--------|------|--------|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1264 | 1221 | 1209 | 1197 | 1186 | 1177 | 1165 | 1153 | 1142 | 1142 | 1131 | 988 |
| Nominal Axial Capacity (kips per pile) | 4606.1 | 4238 | 3869.9 | 3501.6 | 3133.3 | 4606.1 | 4238 | 3869.9 | 3501.6 | 2995.4 | 2956.5 | 2544.5 |
| Ultimate Axial Capacity (kips per pile) | 1865 | 1716 | 1567 | 1418 | 1269 | 1865 | 1716 | 1567 | 1418 | 1213 | 1197 | 1031 |
| D/C Ratio | 0.68 | 0.71 | 0.77 | 0.84 | 0.93 | 0.63 | 0.68 | 0.74 | 0.81 | 0.94 | 0.94 | 0.96 |

Factored

From Geotech

including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 1- Section C TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 36-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 1- Section C TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -49.9 | -46.7 | -43.5 | -40.3 |
| Top of BCF Elevation (ft) | -51.0 | -51.0 | -51.0 | -51.0 |
| Top of Silt/Mud (ft) | -51.0 | -51.0 | -51.0 | -51.0 |
| Thickness of Backfill (ft) | 1.1 | 4.3 | 7.5 | 10.7 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Pile Tip Elevation (ft) | -145.0 | -145.0 | -140.0 | -140.0 |
| Total Pile Length (ft) | 178.0 | 178.5 | 173.5 | 173.5 |
| Pile Length Above Ground Surface (ft) | 82.9 | 80.2 | 77.0 | 73.8 |
| Pile Embedment in the Soil (ft) | 95.1 | 98.3 | 96.5 | 99.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 250.2 | 245.6 | 237.2 | 231.4 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1067 | 1089 | 1079 | 1008 |
| Nominal Axial Capacity (kips per pile) | 3133.9 | 3518.6 | 3397.4 | 3579.9 |
| Ultimate Axial Capacity (kips per pile) | 1128 | 1267 | 1223 | 1289 |
| D/C Ratio | 0.95 | 0.86 | 0.88 | 0.78 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5 - Section B Trestle - 48" Hybrid Pile

By: **G. Jankhah**
 Date: **January 29, 2013**
 Checked By: **Vu Phan**
 Date: **February 11, 2013**

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section B Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 14 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 182 |
| Ground Elevation (ft) | -27.4 | -23.4 | -19.4 | -15.4 | -11.4 | -7.4 | -3.4 | 0.6 | 4.6 | 7.4 |
| Top of BCF Elevation (ft) | -27.4 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 | -25.0 |
| Top of Silt/Mud (ft) | -27.4 | -23.4 | -19.4 | -15.4 | -11.4 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 2.6 | 6.6 | 10.6 | 14.6 | 17.4 |
| Thickness of Soft Mud (ft) | 0.0 | 1.6 | 5.6 | 9.6 | 13.6 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| Pile Tip Elevation (ft) | -165.0 | -160.0 | -155.0 | -150.0 | -145.0 | -140.0 | -140.0 | -140.0 | -125.0 | -110.0 |
| Total Pile Length (ft) | 198.0 | 193.5 | 188.5 | 183.5 | 178.5 | 173.5 | 173.5 | 173.5 | 158.5 | 143.5 |
| Pile Length Above Ground Surface (ft) | 60.4 | 56.9 | 52.9 | 48.9 | 44.9 | 40.9 | 36.9 | 32.9 | 28.9 | 26.1 |
| Pile Embedment in the Soil (ft) | 137.6 | 136.6 | 135.6 | 134.6 | 133.6 | 132.6 | 136.6 | 140.6 | 129.6 | 117.4 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.75 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing
 Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 | 45.0 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 25.5 | 15.0 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 72.9 |
| Crane Rail Beam | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 219.8 | 211.2 | 201.4 | 191.6 | 181.8 | 172.0 | 164.7 | 157.5 | 142.6 | 129.8 |
| 8" dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 | 200.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | |
|---|--------|--------|--------|--------|--------|------|--------|--------|--------|------|
| Total Axial Demand (kips per pile) | 1259 | 1217 | 1205 | 1194 | 1182 | 1170 | 1162 | 1153 | 1017 | 705 |
| Nominal Axial Capacity (kips per pile) | 4831.8 | 4462.7 | 4093.5 | 3724.1 | 3354.6 | 2985 | 4462.7 | 3058.4 | 2641.2 | 2220 |
| Ultimate Axial Capacity (kips per pile) | 1957 | 1807 | 1658 | 1508 | 1359 | 1209 | 1807 | 1239 | 1070 | 749 |
| D/C Ratio | 0.64 | 0.67 | 0.73 | 0.79 | 0.87 | 0.97 | 0.64 | 0.93 | 0.95 | 0.94 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5 - Section B BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section B BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|-------------------|--------|--------|------------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -39.9 | -36.7 | -33.5 | -30.3 |
| Top of BCF Elevation (ft) | -39.9 | -36.7 | -33.5 | -30.3 |
| Top of Silt/Mud (ft) | -39.9 | -36.7 | -33.5 | -30.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Pile Tip Elevation (ft) | -145.0 | -145.0 | -140.0 | -135.0 |
| Total Pile Length (ft) | 178.0 | 178.5 | 173.5 | 168.5 |
| Pile Length Above Ground Surface (ft) | 72.9 | 70.2 | 67.0 | 63.8 |
| Pile Embedment in the Soil (ft) | 105.1 | 108.3 | 106.5 | 104.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 232.1 | 227.5 | 219.1 | 210.8 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|--------|--------|------|------|
| Total Axial Demand (kips per pile) | 1046 | 1067 | 1057 | 983 |
| Nominal Axial Capacity (kips per pile) | 3402.1 | 4044.2 | 3769 | 3035 |
| Ultimate Axial Capacity (kips per pile) | 1225 | 1456 | 1357 | 1093 |
| D/C Ratio | 0.85 | 0.73 | 0.78 | 0.90 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5 - Section A Trestle - 48" Hybrid Pile

By: G. Jankhah
Date: January 29, 2013
Checked By: Vu Phan
Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section A Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 14 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 182 |
| Ground Elevation (ft) | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 | 10.6 | 14.6 | 17.4 |
| Top of BCF Elevation (ft) | -17.4 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 | -15.0 |
| Top of Silt/Mud (ft) | -17.4 | -13.4 | -9.4 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 | -10.0 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 4.6 | 8.6 | 12.6 | 16.6 | 20.6 | 24.6 | 27.4 |
| Thickness of Soft Mud (ft) | 0.0 | 1.6 | 5.6 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Pile Tip Elevation (ft) | -155.0 | -150.0 | -145.0 | -140.0 | -135.0 | -130.0 | -125.0 | -125.0 | -115.0 | -100.0 |
| Total Pile Length (ft) | 188.0 | 183.5 | 178.5 | 173.5 | 168.5 | 163.5 | 158.5 | 158.5 | 148.5 | 133.5 |
| Pile Length Above Ground Surface (ft) | 50.4 | 46.9 | 42.9 | 38.9 | 34.9 | 30.9 | 26.9 | 22.9 | 18.9 | 16.1 |
| Pile Embedment in the Soil (ft) | 137.6 | 136.6 | 135.6 | 134.6 | 133.6 | 132.6 | 131.6 | 135.6 | 129.6 | 117.4 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.75 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 | 45.0 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 25.5 | 15.0 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 72.9 |
| Crane Rail Beam | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 196.6 | 188.0 | 178.2 | 168.4 | 158.6 | 148.8 | 139.0 | 131.8 | 119.4 | 106.6 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 | 200.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | |
|---|----------|-----------|----------|----------|----------|------|------|--------|--------|------|
| Total Axial Demand (kips per pile) | 1231 | 1189 | 1178 | 1166 | 1154 | 1142 | 1131 | 1122 | 989 | 677 |
| Nominal Axial Capacity (kips per pile) | 3172.612 | 3134.7139 | 3096.816 | 3058.919 | 3021.021 | 2985 | 2946 | 2865.1 | 2641.2 | 2220 |
| Ultimate Axial Capacity (kips per pile) | 1285 | 1270 | 1254 | 1239 | 1224 | 1209 | 1193 | 1160 | 1070 | 749 |
| D/C Ratio | 0.96 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.95 | 0.97 | 0.93 | 0.90 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5 - Section A BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section A BARGE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|-------------------|--------|--------|------------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -29.9 | -26.7 | -23.5 | -20.3 |
| Top of BCF Elevation (ft) | -29.9 | -26.7 | -23.5 | -20.3 |
| Top of Silt/Mud (ft) | -29.9 | -26.7 | -23.5 | -20.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Pile Tip Elevation (ft) | -145.0 | -145.0 | -140.0 | -135.0 |
| Total Pile Length (ft) | 178.0 | 178.5 | 173.5 | 168.5 |
| Pile Length Above Ground Surface (ft) | 62.9 | 60.2 | 57.0 | 53.8 |
| Pile Embedment in the Soil (ft) | 115.1 | 118.3 | 116.5 | 114.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 214.0 | 209.4 | 201.1 | 192.7 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|---------|---------|---------|---------|
| Total Axial Demand (kips per pile) | 1024 | 1045 | 1035 | 962 |
| Nominal Axial Capacity (kips per pile) | 3405.33 | 4052.81 | 3773.79 | 3036.97 |
| Ultimate Axial Capacity (kips per pile) | 1226 | 1459 | 1359 | 1093 |
| D/C Ratio | 0.84 | 0.72 | 0.76 | 0.88 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section C Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|------|------------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.24 | (1V:4.25H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section C Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 188 |
| Ground Elevation (ft) | -35.0 | -30.3 | -25.6 | -20.9 | -16.2 | -11.5 | -6.8 | -2.1 | 2.6 | 7.3 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -35.0 | -30.3 | -25.6 | -20.9 | -16.2 | -11.5 | -6.8 | -2.1 | 2.6 | 7.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 20.0 | 24.7 | 29.4 | 34.1 | 38.8 | 43.5 | 48.2 | 52.9 | 57.6 | 62.3 |
| Pile Tip Elevation (ft) | -170.0 | -160.0 | -155.0 | -150.0 | -145.0 | -140.0 | -135.0 | -130.0 | -125.0 | -95.0 |
| Total Pile Length (ft) | 203.0 | 193.5 | 188.5 | 183.5 | 178.5 | 173.5 | 168.5 | 163.5 | 158.5 | 128.5 |
| Pile Length Above Ground Surface (ft) | 68.0 | 63.8 | 59.1 | 54.4 | 49.7 | 45.0 | 40.3 | 35.6 | 30.9 | 26.2 |
| Pile Embedment in the Soil (ft) | 135.0 | 129.7 | 129.4 | 129.1 | 128.8 | 128.5 | 128.2 | 127.9 | 127.6 | 102.3 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 58.5 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 19.5 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 72.9 |
| Crane Rail Beam | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 236.1 | 223.6 | 212.6 | 201.5 | 190.4 | 179.4 | 168.3 | 157.2 | 146.2 | 122.3 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 260.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | |
|---|------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1278 | 1232 | 1219 | 1206 | 1192 | 1179 | 1166 | 1152 | 1139 | 813 |
| Nominal Axial Capacity (kips per pile) | 3327 | 3124.3 | 3124.3 | 3124.3 | 3084.5 | 3084.5 | 3084.5 | 3044.9 | 3044.9 | 2132.5 |
| Ultimate Axial Capacity (kips per pile) | 1347 | 1265 | 1265 | 1265 | 1249 | 1249 | 1249 | 1233 | 1233 | 864 |
| D/C Ratio | 0.95 | 0.97 | 0.96 | 0.95 | 0.95 | 0.94 | 0.93 | 0.93 | 0.92 | 0.94 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5 - Section C TOTE Berth - 48" Hybrid Pile

By: **G. Jankhah**
 Date: **January 29, 2013**
 Checked By: **Vu Phan**
 Date: **February 11, 2013**

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|------|------------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.24 | (1V:4.25H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section C TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -49.7 | -45.9 | -42.2 | -38.4 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -49.7 | -45.9 | -42.2 | -38.4 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 5.3 | 9.1 | 12.8 | 16.6 |
| Pile Tip Elevation (ft) | -170.0 | -165.0 | -165.0 | -160.0 |
| Total Pile Length (ft) | 203.0 | 198.5 | 198.5 | 193.5 |
| Pile Length Above Ground Surface (ft) | 82.7 | 79.4 | 75.7 | 71.9 |
| Pile Embedment in the Soil (ft) | 120.3 | 119.1 | 122.8 | 121.6 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 262.7 | 254.5 | 247.7 | 238.3 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1082 | 1099 | 1091 | 1016 |
| Nominal Axial Capacity (kips per pile) | 4055.7 | 3692.5 | 4781.6 | 4418.7 |
| Ultimate Axial Capacity (kips per pile) | 1460 | 1329 | 1721 | 1591 |
| D/C Ratio | 0.74 | 0.83 | 0.63 | 0.64 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5 - Section D Trestle - 48" Hybrid Pile

By: G. Jankhah
Date: January 29, 2013
Checked By: Vu Phan
Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|------|------------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.24 | (1V:4.25H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section D Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 |
| Ground Elevation (ft) | -21.8 | -17.1 | -12.4 | -7.7 | -3.0 | 1.7 | 6.4 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -21.8 | -17.1 | -12.4 | -7.7 | -3.0 | 1.7 | 6.4 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 33.2 | 37.9 | 42.6 | 47.3 | 52.0 | 56.7 | 61.4 |
| Pile Tip Elevation (ft) | -160.0 | -150.0 | -145.0 | -140.0 | -135.0 | -130.0 | -115.0 |
| Total Pile Length (ft) | 193.0 | 183.5 | 178.5 | 173.5 | 168.5 | 163.5 | 148.5 |
| Pile Length Above Ground Surface (ft) | 54.8 | 50.6 | 45.9 | 41.2 | 36.5 | 31.8 | 27.1 |
| Pile Embedment in the Soil (ft) | 138.2 | 132.9 | 132.6 | 132.3 | 132.0 | 131.7 | 121.4 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 25.5 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 83.7 |
| Crane Rail Beam | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 207.1 | 194.7 | 183.6 | 172.5 | 161.5 | 150.4 | 134.2 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | |
|------------|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | |
|------------|--|--|--|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | |
|---|--------|--------|--------|--------|--------|--------|------|
| Total Axial Demand (kips per pile) | 1244 | 1197 | 1184 | 1171 | 1158 | 1144 | 998 |
| Nominal Axial Capacity (kips per pile) | 3302.9 | 3060.5 | 3060.5 | 3060.5 | 3060.5 | 3020.9 | 2638 |
| Ultimate Axial Capacity (kips per pile) | 1338 | 1240 | 1240 | 1240 | 1240 | 1223 | 1068 |
| D/C Ratio | 0.93 | 0.97 | 0.96 | 0.94 | 0.93 | 0.94 | 0.93 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section D Horizon Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|------|------------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 18 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | 10 | ft | |
| Embankment Slope | 0.24 | (1V:4.25H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5.25 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5 - Section D Horizon Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Water-side Crane Rail | | | | | | Land-side Crane Rail |
|--|--------------------------|--------|--------|--------|--------|--------|-------------------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| Distance to Previous Row (ft) | 0 | 12 | 18 | 18 | 18 | 18 | 16 |
| Distance to Water-side Crane Rail (ft) | 0 | 12 | 30 | 48 | 66 | 84 | 100 |
| Distance to Wharf Face (ft) | 8 | 20 | 38 | 56 | 74 | 92 | 108 |
| Ground Elevation (ft) | -49.1 | -46.3 | -42.1 | -37.8 | -33.6 | -29.4 | -25.6 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -49.1 | -46.3 | -42.1 | -37.8 | -33.6 | -29.4 | -25.6 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Silt/Mud (ft) | 5.9 | 8.7 | 12.9 | 17.2 | 21.4 | 25.6 | 29.4 |
| Pile Tip Elevation (ft) | -190.0 | -180.0 | -170.0 | -165.0 | -160.0 | -155.0 | -165.0 |
| Total Pile Length (ft) | 222.8 | 213.5 | 203.5 | 198.5 | 193.5 | 188.5 | 198.5 |
| Pile Length Above Ground Surface (ft) | 81.9 | 79.8 | 75.6 | 71.3 | 67.1 | 62.9 | 59.1 |
| Pile Embedment in the Soil (ft) | 140.9 | 133.7 | 127.9 | 127.2 | 126.4 | 125.6 | 139.4 |
| Group Reduction Factor | 0.65 | 0.70 | 0.85 | 0.85 | 0.85 | 0.80 | 0.65 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | | 67.5 | 81.0 | 81.0 | 81.0 | 76.5 | |
| 6" Asphalt Overlay | | 22.5 | 27.0 | 27.0 | 27.0 | 25.5 | |
| Pile Cap | | 86.4 | 86.4 | 86.4 | 86.4 | 86.4 | |
| Crane Rail Beam | 94.5 | | | | | | 94.5 |
| 48" Steel Pipe Pile (including Concrete Fill) | 271.3 | 262.8 | 250.0 | 239.8 | 229.6 | 219.3 | 217.7 |
| 8' dia. Ice | 56.9 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | |
|-------------------|--|-------|-------|-------|-------|-------|--|
| Uniform Live Load | | 300.0 | 360.0 | 360.0 | 360.0 | 340.0 | |
|-------------------|--|-------|-------|-------|-------|-------|--|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | |
|------------|-------|--|--|--|--|--|-------|
| Crane Rail | 500.0 | | | | | | 500.0 |
|------------|-------|--|--|--|--|--|-------|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | |
|---|--------|--------|--------|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1157 | 1077 | 1179 | 1167 | 1154 | 1103 | 1094 |
| Nominal Axial Capacity (kips per pile) | 4855.6 | 3597.6 | 3345.9 | 3345.9 | 3304.9 | 3264.1 | 4493.9 |
| Ultimate Axial Capacity (kips per pile) | 1420 | 1133 | 1280 | 1280 | 1264 | 1175 | 1314 |
| D/C Ratio | 0.81 | 0.95 | 0.92 | 0.91 | 0.91 | 0.94 | 0.83 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5H - Section A Access Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section A Access Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | | | | | | | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 188 | 208 | 228 |
| Ground Elevation (ft) | -37.4 | -33.4 | -29.4 | -25.4 | -21.4 | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 |
| Top of BCF Elevation (ft) | -45.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 | -41.0 |
| Top of Silt/Mud (ft) | -45.0 | -41.0 | -41.0 | -36.0 | -31.0 | -26.0 | -21.0 | -19.0 | -19.0 | -19.0 | -19.0 | -19.0 |
| Thickness of Backfill (ft) | 7.6 | 7.6 | 11.6 | 10.6 | 9.6 | 8.6 | 7.6 | 9.6 | 13.6 | 17.6 | 21.6 | 25.6 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 5.0 | 10.0 | 15.0 | 20.0 | 22.0 | 22.0 | 22.0 | 22.0 | 22.0 |
| Pile Tip Elevation (ft) | -165.0 | -160.0 | -155.0 | -150.0 | -145.0 | -145.0 | -140.0 | -135.0 | -130.0 | -145.0 | -140.0 | -130.0 |
| Total Pile Length (ft) | 198.0 | 193.5 | 188.5 | 183.5 | 178.5 | 178.5 | 173.5 | 168.5 | 163.5 | 178.5 | 173.5 | 163.5 |
| Pile Length Above Ground Surface (ft) | 70.4 | 66.9 | 62.9 | 58.9 | 54.9 | 50.9 | 46.9 | 42.9 | 38.9 | 34.9 | 30.9 | 26.9 |
| Pile Embedment in the Soil (ft) | 127.6 | 126.6 | 125.6 | 124.6 | 123.6 | 127.6 | 126.6 | 125.6 | 124.6 | 143.6 | 142.6 | 136.6 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |

Note

Based on 5:1 Slope

Estimated, based on North Extension As-built Drawing

Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 76.5 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 25.5 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 83.7 |
| Crane Rail Beam | | | | | | | | | | | | | |
| 36" Steel Pipe Pile (including Concrete Fill) | 237.9 | 229.2 | 219.4 | 209.6 | 199.8 | 192.6 | 182.8 | 173.0 | 163.2 | 163.7 | 153.9 | 141.5 | |
| 8" dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 340.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|--|--|--|

50 kif, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | | | | | | |
|---|--------|------|--------|--------|--------|--------|------|--------|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1280 | 1239 | 1227 | 1215 | 1204 | 1195 | 1183 | 1171 | 1160 | 1160 | 1149 | 1006 |
| Nominal Axial Capacity (kips per pile) | 4606.1 | 4238 | 3869.9 | 3501.6 | 3133.3 | 4606.1 | 4238 | 3869.9 | 3501.6 | 2995.4 | 2956.5 | 2728.2 |
| Ultimate Axial Capacity (kips per pile) | 1865 | 1716 | 1567 | 1418 | 1269 | 1865 | 1716 | 1567 | 1418 | 1213 | 1197 | 1105 |
| D/C Ratio | 0.69 | 0.72 | 0.78 | 0.86 | 0.95 | 0.64 | 0.69 | 0.75 | 0.82 | 0.96 | 0.96 | 0.91 |

Factored

From Geotech

including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5H - Section A TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section A TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|----------------|--------|--------|---------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -49.9 | -46.7 | -43.5 | -40.3 |
| Top of BCF Elevation (ft) | -51.0 | -51.0 | -51.0 | -51.0 |
| Top of Silt/Mud (ft) | -51.0 | -51.0 | -51.0 | -51.0 |
| Thickness of Backfill (ft) | 1.1 | 4.3 | 7.5 | 10.7 |
| Thickness of Soft Mud (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Pile Tip Elevation (ft) | -145.0 | -145.0 | -140.0 | -140.0 |
| Total Pile Length (ft) | 178.0 | 178.5 | 173.5 | 173.5 |
| Pile Length Above Ground Surface (ft) | 82.9 | 80.2 | 77.0 | 73.8 |
| Pile Embedment in the Soil (ft) | 95.1 | 98.3 | 96.5 | 99.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 36" Steel Pipe Pile (including Concrete Fill) | 250.2 | 245.6 | 237.2 | 231.4 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1067 | 1089 | 1079 | 1008 |
| Nominal Axial Capacity (kips per pile) | 3133.9 | 3518.6 | 3397.4 | 3579.9 |
| Ultimate Axial Capacity (kips per pile) | 1128 | 1267 | 1223 | 1289 |
| D/C Ratio | 0.95 | 0.86 | 0.88 | 0.78 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5H - Section B Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section B Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

| Pile Data | | | | | | | | | | | | | | | | | | | |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| | Front Row Pile | | | | | | | | | | | | | | | | | | Back-row Pile |
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 188 | 208 | 228 | 248 | 268 | 288 | 308 | 328 | 348 | 368 |
| Ground Elevation (ft) | -37.4 | -33.4 | -29.4 | -25.4 | -21.4 | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 | 10.6 | 14.6 | 18.6 | 22.6 | 26.6 | 30.6 | 34.6 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -37.4 | -33.4 | -29.4 | -25.4 | -21.4 | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 | 10.6 | 14.6 | 18.6 | 22.6 | 26.6 | 30.6 | 34.6 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 17.6 | 21.6 | 25.6 | 29.6 | 33.6 | 37.6 | 41.6 | 45.6 | 49.6 | 53.6 | 57.6 | 61.6 | 65.6 | 69.6 | 73.6 | 77.6 | 81.6 | 85.6 | 89.6 |
| Pile Tip Elevation (ft) | -175.0 | -170.0 | -165.0 | -160.0 | -155.0 | -150.0 | -145.0 | -140.0 | -135.0 | -130.0 | -125.0 | -135.0 | -130.0 | -125.0 | -120.0 | -115.0 | -110.0 | -105.0 | -105.0 |
| Total Pile Length (ft) | 208.0 | 203.5 | 198.5 | 193.5 | 188.5 | 183.5 | 178.5 | 173.5 | 168.5 | 163.5 | 158.5 | 168.5 | 163.5 | 158.5 | 153.5 | 148.5 | 143.5 | 138.5 | 138.5 |
| Pile Length Above Ground Surface (ft) | 70.4 | 66.9 | 62.9 | 58.9 | 54.9 | 50.9 | 46.9 | 42.9 | 38.9 | 34.9 | 30.9 | 26.9 | 22.9 | 18.9 | 14.9 | 10.9 | 6.9 | 2.9 | -1.1 |
| Pile Embedment in the Soil (ft) | 137.6 | 136.6 | 135.6 | 134.6 | 133.6 | 132.6 | 131.6 | 130.6 | 129.6 | 128.6 | 127.6 | 141.6 | 140.6 | 139.6 | 138.6 | 137.6 | 136.6 | 135.6 | 139.6 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |

Dead Load (kips per pile)

| | | | | | | | | | | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|------|------|------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 99.0 |
| 6" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 33.0 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 97.2 |
| Crane Rail Beam | | | | | | | | | | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 243.0 | 234.4 | 224.6 | 214.8 | 205.0 | 195.2 | 185.4 | 175.6 | 165.8 | 156.0 | 146.2 | 144.1 | 134.3 | 124.5 | 114.7 | 104.9 | 95.1 | 85.3 | 78.1 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | | | | | | | | | | | | | |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 440.0 |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|

Crane Rail Load (kips per pile)

| | | | | | | | | | | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | | | | | | | | | | |
|------------|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|

Demand vs. Capacity

| | | | | | | | | | | | | | | | | | | | |
|---|--------|--------|--------|------|------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|------|--------|--------|
| Total Axial Demand (kips per pile) | 1287 | 1245 | 1233 | 1222 | 1210 | 1198 | 1186 | 1175 | 1163 | 1151 | 1139 | 1137 | 1125 | 1113 | 1102 | 1090 | 1078 | 1066 | 1142 |
| Nominal Axial Capacity (kips per pile) | 3451.5 | 3409.8 | 3368.3 | 3327 | 3286 | 3245.2 | 3204.7 | 3164.4 | 3124.3 | 3084.5 | 3044.9 | 3069.4 | 3029.7 | 2990.2 | 2950.9 | 2911.8 | 2873 | 2834.4 | 2990.2 |
| Ultimate Axial Capacity (kips per pile) | 1398 | 1381 | 1364 | 1347 | 1331 | 1314 | 1298 | 1282 | 1265 | 1249 | 1233 | 1243 | 1227 | 1211 | 1195 | 1179 | 1164 | 1148 | 1211 |
| D/C Ratio | 0.92 | 0.90 | 0.90 | 0.91 | 0.91 | 0.91 | 0.91 | 0.92 | 0.92 | 0.92 | 0.92 | 0.91 | 0.92 | 0.92 | 0.92 | 0.92 | 0.93 | 0.93 | 0.94 |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section B TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 16 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section B TOTE Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Front Row Pile | | | Back-row Pile |
|---------------------------------------|-------------------|--------|--------|------------------|
| Row | 1 | 2 | 3 | 4 |
| Distance to Previous Row (ft) | 0 | 16 | 16 | 16 |
| Distance to Wharf Face (ft) | 5.5 | 21.5 | 37.5 | 53.5 |
| Ground Elevation (ft) | -49.9 | -46.7 | -43.5 | -40.3 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -49.9 | -46.7 | -43.5 | -40.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 5.1 | 8.3 | 11.5 | 14.7 |
| Pile Tip Elevation (ft) | -170.0 | -165.0 | -165.0 | -160.0 |
| Total Pile Length (ft) | 203.0 | 198.5 | 198.5 | 193.5 |
| Pile Length Above Ground Surface (ft) | 82.9 | 80.2 | 77.0 | 73.8 |
| Pile Embedment in the Soil (ft) | 120.1 | 118.3 | 121.5 | 119.7 |
| Group Reduction Factor | 0.80 | 0.80 | 0.80 | 0.80 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | |
|---|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | 60.8 | 72.0 | 72.0 | 65.3 |
| 6" Asphalt Overlay | 20.3 | 24.0 | 24.0 | 21.8 |
| Pile Cap | 141.0 | 81.0 | 81.0 | 77.0 |
| Crane Rail Beam | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 263.0 | 255.8 | 250.0 | 241.7 |
| 8' dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | |
|-------------------|-------|-------|-------|-------|
| Uniform Live Load | 270.0 | 320.0 | 320.0 | 290.0 |
|-------------------|-------|-------|-------|-------|

1000 psf

Crane Rail Load (kips per pile)

| | | | | |
|------------|--|--|--|--|
| Crane Rail | | | | |
|------------|--|--|--|--|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | |
|---|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1083 | 1101 | 1094 | 1020 |
| Nominal Axial Capacity (kips per pile) | 4055.7 | 3329.3 | 4418.7 | 3692.5 |
| Ultimate Axial Capacity (kips per pile) | 1460 | 1199 | 1591 | 1329 |
| D/C Ratio | 0.74 | 0.92 | 0.69 | 0.77 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Pile Axial Capacity Design - Option 5H - Section C Trestle - 48" Hybrid Pile

By: G. Jankhah
Date: January 29, 2013
Checked By: Vu Phan
Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 20 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | N/A | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|--------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft^2 | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch^2 | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch^2 | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section C Trestle - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

| Pile Data | | | | | | | | | | | | | | | | | |
|---------------------------------------|----------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| | Front Row Pile | | | | | | | | | | | | | | | | Back-row Pile |
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 |
| Distance to Previous Row (ft) | 0 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 12 |
| Distance to Wharf Face (ft) | 8 | 28 | 48 | 68 | 88 | 108 | 128 | 148 | 168 | 188 | 208 | 228 | 248 | 268 | 288 | 308 | 320 |
| Ground Elevation (ft) | -26.2 | -22.2 | -18.2 | -14.2 | -10.2 | -6.2 | -2.2 | 1.8 | 5.8 | 9.8 | 13.8 | 17.8 | 21.8 | 25.8 | 29.8 | 33.8 | 36.2 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -26.2 | -22.2 | -18.2 | -14.2 | -10.2 | -6.2 | -2.2 | 1.8 | 5.8 | 9.8 | 13.8 | 17.8 | 21.8 | 25.8 | 29.8 | 33.8 | 36.2 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Soft Mud (ft) | 28.8 | 32.8 | 36.8 | 40.8 | 44.8 | 48.8 | 52.8 | 56.8 | 60.8 | 64.8 | 68.8 | 72.8 | 76.8 | 80.8 | 84.8 | 88.8 | 91.2 |
| Pile Tip Elevation (ft) | -165.0 | -160.0 | -155.0 | -150.0 | -145.0 | -140.0 | -135.0 | -130.0 | -125.0 | -140.0 | -135.0 | -130.0 | -125.0 | -120.0 | -115.0 | -115.0 | -90.0 |
| Total Pile Length (ft) | 198.0 | 193.5 | 188.5 | 183.5 | 178.5 | 173.5 | 168.5 | 163.5 | 158.5 | 173.5 | 168.5 | 163.5 | 158.5 | 153.5 | 148.5 | 148.5 | 123.5 |
| Pile Length Above Ground Surface (ft) | 59.2 | 55.7 | 51.7 | 47.7 | 43.7 | 39.7 | 35.7 | 31.7 | 27.7 | 23.7 | 19.7 | 15.7 | 11.7 | 7.7 | 3.7 | -0.3 | -2.7 |
| Pile Embedment in the Soil (ft) | 138.8 | 137.8 | 136.8 | 135.8 | 134.8 | 133.8 | 132.8 | 131.8 | 130.8 | 149.8 | 148.8 | 147.8 | 146.8 | 145.8 | 144.8 | 148.8 | 126.2 |
| Group Reduction Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.70 | 0.70 |

| Dead Load (kips per pile) | | | | | | | | | | | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|------|------|------|
| 24" Precast Haunched Deck Panel | 81.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 90.0 | 72.0 | 45.0 |
| 3" Asphalt Overlay | 27.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 24.0 | 15.0 |
| Pile Cap | 183.9 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 91.8 | 75.6 |
| Crane Rail Beam | | | | | | | | | | | | | | | | | |
| 48" Steel Pipe Pile (including Concrete Fill) | 217.6 | 209.0 | 199.2 | 189.4 | 179.6 | 169.8 | 160.0 | 150.2 | 140.4 | 140.9 | 131.1 | 121.3 | 111.5 | 101.7 | 91.9 | 84.7 | 67.5 |
| 8" dia. Ice | 57.3 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

| Live Load (kips per pile) | | | | | | | | | | | | | | | | | |
|---------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Uniform Live Load | 360.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 400.0 | 320.0 | 200.0 |

| Crane Rail Load (kips per pile) | | | | | | | | | | | | | | | | | |
|---------------------------------|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|--|
| Crane Rail | | | | | | | | | | | | | | | | | |

| Demand vs. Capacity | | | | | | | | | | | | | | | | | |
|---|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|------|--------|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1256 | 1215 | 1203 | 1191 | 1179 | 1168 | 1156 | 1144 | 1132 | 1133 | 1121 | 1109 | 1098 | 1086 | 1074 | 909 | 633 |
| Nominal Axial Capacity (kips per pile) | 3302.9 | 3261.9 | 3221.2 | 3180.6 | 3140.3 | 3100.3 | 3060.5 | 3020.9 | 2981.5 | 3058.8 | 3019.3 | 2980 | 2940.9 | 2902.1 | 2863.5 | 3019.3 | 2208.5 |
| Ultimate Axial Capacity (kips per pile) | 1338 | 1321 | 1305 | 1288 | 1272 | 1256 | 1240 | 1223 | 1208 | 1239 | 1223 | 1207 | 1191 | 1175 | 1160 | 951 | 696 |
| D/C Ratio | 0.94 | 0.92 | 0.92 | 0.92 | 0.93 | 0.93 | 0.93 | 0.94 | 0.94 | 0.91 | 0.92 | 0.92 | 0.92 | 0.92 | 0.93 | 0.96 | 0.91 |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section C Horizon Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Purpose of Calculations:

To evaluate the axial capacity of 48-in hybrid pipe piles under the design loading and verify the pile embedments.

References:

1. AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications, Sixth Edition with Interims, 2012
2. Port of Anchorage 15% Concept Plans
3. LBFoster Pipe Piling
4. Structural Design Criteria
5. Geotechnical Memorandum

Design Assumptions:

General Wharf Configuration

| | | | |
|--------------------------------------|-----|---------|----------------|
| Typical Pile Longitudinal Spacing | 20 | ft | Per Plan Sheet |
| Typical Pile Transverse Spacing | 18 | ft | Per Plan Sheet |
| Crane Rail Pile Longitudinal Spacing | 10 | ft | |
| Embankment Slope | 0.2 | (1V:5H) | Per Plan Sheet |
| Deck Elevation | 38 | ft | Per Plan Sheet |
| Low Water Elevation (Bot. Ice) | -5 | ft | Per Plan Sheet |

Structural Dimensions

| | | | |
|---------------------------------|------|-------------------|--|
| Deck Slab Thickness | 18 | inch | Precast Haunched Deck Panel |
| Asphalt Overlay Thickness | 6 | inch | Per Plan Sheet |
| Pile Cap Width | 6 | ft | Per Plan Sheet |
| Pile Cap Depth | 3 | ft | Per Plan Sheet |
| Pile Cap Depth at Face of Wharf | 5.25 | ft | Per Plan Sheet |
| Crane Rail Beam Size | 63 | ft ² | 5.25' (D) x 12' (W) including deck; Per Plan Sheet |
| Pile Diameter | 48 | inch | Steel Pipe Pile; Per Plan Sheet |
| Pile Wall Thickness | 1 | inch | Steel Pipe Pile Only; Per Plan Sheet |
| Pile Area | 148 | inch ² | |
| Pile Weight | 513 | lbs/ft | LBFoster Pipe Piling |
| Inside Concrete Fill Area | 1735 | inch ² | Steel Hybrid Pile Only |
| Ice Diameter | 8 | ft | Per Structural Design Criteria |

Material Properties

| | | | |
|-----------------------------------|------|-----|--------------------------------|
| Unit Weight - Reinforced Concrete | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Structural Steel | 0.49 | kcf | Per Structural Design Criteria |
| Unit Weight - Asphalt Paving | 0.15 | kcf | Per Structural Design Criteria |
| Unit Weight - Ice | 0.04 | kcf | Per Structural Design Criteria |

Load and Resistance Factors - LRFD

| | | | |
|-------------------------|------|--|---|
| Dead Load Factor | 1.2 | | Static Case; Per Structural Design Criteria |
| Live Load Factor | 1.6 | | Static Case; Per Structural Design Criteria |
| Crane Rail Load Factor | 1.3 | | Per Structural Design Criteria |
| Axial Resistance Factor | 0.45 | | for Driven Pile |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Pile Axial Capacity Design - Option 5H - Section C Horizon Berth - 48" Hybrid Pile

By: G. Jankhah
 Date: January 29, 2013
 Checked By: Vu Phan
 Date: February 11, 2013

Pile Data

| | Water-side Crane Rail | | | | | | Land-side Crane Rail |
|--|--------------------------|--------|--------|--------|--------|--------|-------------------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| Distance to Previous Row (ft) | 0 | 12 | 18 | 18 | 18 | 18 | 16 |
| Distance to Water-side Crane Rail (ft) | 0 | 12 | 30 | 48 | 66 | 84 | 100 |
| Distance to Wharf Face (ft) | 8.5 | 20.5 | 38.5 | 56.5 | 74.5 | 92.5 | 108.5 |
| Ground Elevation (ft) | -49.3 | -46.9 | -43.3 | -39.7 | -36.1 | -32.5 | -29.3 |
| Top of BCF Elevation (ft) | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 | -55.0 |
| Top of Silt/Mud (ft) | -49.3 | -46.9 | -43.3 | -39.7 | -36.1 | -32.5 | -29.3 |
| Thickness of Backfill (ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Thickness of Silt/Mud (ft) | 5.7 | 8.1 | 11.7 | 15.3 | 18.9 | 22.5 | 25.7 |
| Pile Tip Elevation (ft) | -190.0 | -180.0 | -170.0 | -165.0 | -160.0 | -160.0 | -170.0 |
| Total Pile Length (ft) | 222.8 | 213.5 | 203.5 | 198.5 | 193.5 | 193.5 | 203.5 |
| Pile Length Above Ground Surface (ft) | 82.1 | 80.4 | 76.8 | 73.2 | 69.6 | 66.0 | 62.8 |
| Pile Embedment in the Soil (ft) | 140.7 | 133.1 | 126.7 | 125.3 | 123.9 | 127.5 | 140.7 |
| Group Reduction Factor | 0.65 | 0.70 | 0.85 | 0.85 | 0.85 | 0.80 | 0.65 |

Note

Based on 5:1 Slope
 Estimated, based on North Extension As-built Drawing
 Estimated, based on North Extension As-built Drawing

Assume steel pipe pile will be concrete filled above ground surface

Dead Load (kips per pile)

| | | | | | | | |
|---|-------|-------|-------|-------|-------|-------|-------|
| 24" Precast Haunched Deck Panel | | 67.5 | 81.0 | 81.0 | 81.0 | 76.5 | |
| 6" Asphalt Overlay | | 22.5 | 27.0 | 27.0 | 27.0 | 25.5 | |
| Pile Cap | | 86.4 | 86.4 | 86.4 | 86.4 | 86.4 | |
| Crane Rail Beam | 94.5 | | | | | | 94.5 |
| 48" Steel Pipe Pile (including Concrete Fill) | 271.6 | 263.9 | 252.3 | 243.2 | 234.1 | 227.6 | 227.0 |
| 8" dia. Ice | 56.9 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 | 58.1 |

Live Load (kips per pile)

| | | | | | | | |
|-------------------|--|-------|-------|-------|-------|-------|--|
| Uniform Live Load | | 300.0 | 360.0 | 360.0 | 360.0 | 340.0 | |
|-------------------|--|-------|-------|-------|-------|-------|--|

1000 psf

Crane Rail Load (kips per pile)

| | | | | | | | |
|------------|-------|--|--|--|--|--|-------|
| Crane Rail | 500.0 | | | | | | 500.0 |
|------------|-------|--|--|--|--|--|-------|

50 klf, including both dead and live load from crane rail

Demand vs. Capacity

| | | | | | | | |
|---|--------|--------|--------|--------|--------|--------|--------|
| Total Axial Demand (kips per pile) | 1158 | 1078 | 1182 | 1171 | 1160 | 1113 | 1105 |
| Nominal Axial Capacity (kips per pile) | 4855.6 | 3597.6 | 3304.9 | 3264.1 | 3183.2 | 3345.9 | 4855.6 |
| Ultimate Axial Capacity (kips per pile) | 1420 | 1133 | 1264 | 1249 | 1218 | 1205 | 1420 |
| D/C Ratio | 0.82 | 0.95 | 0.93 | 0.94 | 0.95 | 0.92 | 0.78 |

Factored
 From Geotech
 including group reduction factor and resistance factor

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Background: The following summarizes the displacement demand and capacity of the wharf system under seismic demands. The demand to capacity ratio of the structure is verified to be equal to or less than 1.0 for the MCE, CLE, and OLE seismic events.

Design Assumptions: The displacement demand was determined from a linear response spectrum analysis performed on SAP2000 using a 3D model. For the foundation, a point of fixity for each pile was approximated based on LPILE analysis.

The displacement capacity was determined from nonlinear static pushover analysis performed on SAP2000 using 2D models of a critical cross sections. Pile concrete and steel strains were limited based on MOTEMS and POLB seismic criteria and is summarized in Appendix B - Structures Design Criteria. Expected material properties were used for this analysis.

- References:** (1) Marine Oil Terminal Engineering and Maintenance Standards (MOTEMS), California Building Code, Section 3104F, 2011
 (2) Port of Long Beach (POLB) Wharf Design Criteria, Section 4, v 3.0, 2012
 (3) AASHTO LRFD Bridge Design Specifications, Section 10, 6th Edition with Interims, 2012

RC Pile Properties

| | | |
|-------------------------------------|-------|-----|
| Expected Rebar Steel Yield Strength | 66 | ksi |
| Pile Rebar Size | 11 | |
| Pile Rebar Diameter | 1.375 | in |

MOTEMS 3107F.2.1.1
 Analysis is based on 24 #11 bars in connection to pile cap

Steel Pile Properties

| | | |
|-------------------------------|-------|-----------------|
| Expected Steel Yield Strength | 55 | ksi |
| Modulus of Elasticity | 29000 | ksi |
| Pile Diameter | 48 | in |
| Pile Wall Thickness | 1 | in |
| Pile Area | 148 | in ² |
| Pile Moment of Inertia | 40790 | in ⁴ |
| Plastic Modulus | 2209 | in ³ |

MOTEMS 3107F.2.1.1
 Per conceptual plans
 Per conceptual plans; Gross Thickness

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Point of Fixity Computation - Gross Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|-----------------------------------|---------------|--------|--------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Pile Spacing (ft) | 16 | 16 | 16 | 16 |
| Axial Demand (kips) | 200 | 208 | 210 | 213 |
| P-Multiplier | 0.50 | 0.50 | 0.50 | 0.50 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -29.90 | -26.70 | -23.50 | -20.30 |
| Top of Clay Layer | -29.90 | -26.70 | -23.50 | -20.30 |
| Approx. No. Diameters to Fixity | 5 | 5 | 5 | 5 |
| Approx. Point of Fixity Elevation | -49.9 | -46.7 | -43.5 | -40.3 |
| SAP Mudline Shear (kips) | 162.0 | 183.0 | 203.0 | 223.0 |

Assumes gross composite section properties for the pile

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

Point of Fixity Computation - Corroded Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|-----------------------------------|---------------|--------|--------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Pile Spacing (ft) | 16 | 16 | 16 | 16 |
| Axial Demand (kips) | 200 | 208 | 210 | 213 |
| P-Multiplier | 0.50 | 0.50 | 0.50 | 0.50 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -29.90 | -26.70 | -23.50 | -20.30 |
| Top of Clay Layer | -29.90 | -26.70 | -23.50 | -20.30 |
| Approx. No. Diameters to Fixity | 5.5 | 5.5 | 5.5 | 5.5 |
| Approx. Point of Fixity Elevation | -51.9 | -48.7 | -45.5 | -42.3 |
| SAP Mudline Shear (kips) | 127.0 | 145.0 | 163.0 | 183.0 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

P-Delta Check [MOTEMS 3104F.4.3] - Gross Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|--------------------------|---------------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Axial Demand (kips) | 200 | 208 | 210 | 200 |
| Δ_{xx} | 16.6 | 16.6 | 16.6 | 16.6 |
| Δ_{xy} | 2.5 | 2.5 | 2.5 | 2.5 |
| Δ_{yx} | 0 | 0 | 0 | 0 |
| Δ_{yy} | 17.1 | 17.1 | 17.1 | 17.1 |
| Δ_d | 18.53 | 18.53 | 18.53 | 18.53 |
| V/W | 0.81 | 0.88 | 0.97 | 1.12 |
| $4*\Delta_d/H$ | 0.07 | 0.08 | 0.08 | 0.08 |
| P-Delta effect required? | NO | NO | NO | NO |

Assumes gross composite section properties for the pile

From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 for MCE event; MOTEMS 3104F.4.2
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3

P-Delta Check [MOTEMS 3104F.4.3] - Corroded Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|--------------------------|---------------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Axial Demand (kips) | 200 | 208 | 210 | 200 |
| Δ_{xx} | 25.5 | 25.5 | 25.5 | 25.5 |
| Δ_{xy} | 5 | 5 | 5 | 5 |
| Δ_{yx} | 0 | 0 | 0 | 0 |
| Δ_{yy} | 26.6 | 26.6 | 26.6 | 26.6 |
| Δ_d | 29.12 | 29.12 | 29.12 | 29.12 |
| V/W | 0.64 | 0.70 | 0.78 | 0.92 |
| $4*\Delta_d/H$ | 0.11 | 0.12 | 0.12 | 0.13 |
| P-Delta effect required? | NO | NO | NO | NO |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 for MCE event; MOTEMS 3104F.4.2
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Pile Displacement Capacity - Gross Pile Section Properties

| | Demand | Capacity | Check | D/C Ratio |
|-----|--------|----------|-------|-----------|
| OLE | 5 | 15 | GOOD | 0.33 |
| CLE | 12 | 23 | GOOD | 0.52 |
| MCE | 19 | 29 | GOOD | 0.66 |

Assumes gross composite section properties for the pile

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

Pile Displacement Capacity - Corroded Pile Section Properties

| | Demand | Capacity | Check | D/C Ratio |
|-----|--------|----------|-------|-----------|
| OLE | 5 | 22 | GOOD | 0.23 |
| CLE | 13 | 32 | GOOD | 0.41 |
| MCE | 29 | 40 | GOOD | 0.73 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design
Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Wharf

Designed By: V. Phan
Date: February 4, 2013
Checked By: H. Guan
Date: February 11, 2013

Background: The following summarizes the displacement demand and capacity of the wharf system under seismic demands. The demand to capacity ratio of the structure is verified to be equal to or less than 1.0 for the MCE, CLE, and OLE seismic events.

Design Assumptions: The displacement demand was determined from a linear response spectrum analysis performed on SAP2000 using a 3D model. For the foundation, a point of fixity for each pile was approximated based on LPILE analysis.

The displacement capacity was determined from nonlinear static pushover analysis performed on SAP2000 using 2D models of a critical cross sections. Pile concrete and steel strains were limited based on MOTEMS and POLB seismic criteria and is summarized in Appendix B - Structures Design Criteria. Expected material properties were used for this analysis.

References: (1) Marine Oil Terminal Engineering and Maintenance Standards (MOTEMS), California Building Code, Section 3104F, 2011
 (2) Port of Long Beach (POLB) Wharf Design Criteria, Section 4, v 3.0, 2012
 (3) AASHTO LRFD Bridge Design Specifications, Section 10, 6th Edition with Interims, 2012

RC Pile Properties

| | | |
|-------------------------------------|-------|-----|
| Expected Rebar Steel Yield Strength | 66 | ksi |
| Pile Rebar Size | 11 | |
| Pile Rebar Diameter | 1.375 | in |

MOTEMS 3107F.2.1.1
 Analysis is based on 24 #11 bars in connection to pile cap

Steel Pile Properties

| | | |
|-------------------------------|-------|-----------------|
| Expected Steel Yield Strength | 55 | ksi |
| Modulus of Elasticity | 29000 | ksi |
| Pile Diameter | 48 | in |
| Pile Wall Thickness | 1 | in |
| Pile Area | 148 | in ² |
| Pile Moment of Inertia | 40790 | in ⁴ |
| Plastic Modulus | 2209 | in ³ |

MOTEMS 3107F.2.1.1
 Per conceptual plans
 Per conceptual plans; Gross Thickness

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Point of Fixity Computation - Gross Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|--|---------------|--------|--------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Pile Spacing (ft) | 16 | 16 | 16 | 16 |
| Axial Demand (kips) | 200 | 208 | 210 | 213 |
| P-Multiplier | 0.50 | 0.50 | 0.50 | 0.50 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -49.90 | -46.70 | -43.50 | -40.30 |
| Top of Clay Layer | -51.00 | -51.00 | -51.00 | -51.00 |
| Approx. No. Diameters to Fixity ¹ | 5 | 5 | 5 | 5 |
| Approx. Point of Fixity Elevation | -69.9 | -66.7 | -63.5 | -60.3 |
| SAP Mudline Shear (kips) | 141.0 | 157.0 | 171.0 | 184.0 |

Assumes gross composite section properties for the pile

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

Point of Fixity Computation - Corroded Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|--|---------------|--------|--------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Pile Spacing (ft) | 16 | 16 | 16 | 16 |
| Axial Demand (kips) | 200 | 208 | 210 | 213 |
| P-Multiplier | 0.50 | 0.50 | 0.50 | 0.50 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -49.90 | -46.70 | -43.50 | -40.30 |
| Top of Clay Layer | -51.00 | -51.00 | -51.00 | -51.00 |
| Approx. No. Diameters to Fixity ¹ | 4.5 | 4.5 | 4.5 | 4.5 |
| Approx. Point of Fixity Elevation | -67.9 | -64.7 | -61.5 | -58.3 |
| SAP Mudline Shear (kips) | 114.0 | 126.0 | 139.0 | 153.0 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

P-Delta Check [MOTEMS 3104F.4.3] - Gross Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|--------------------------|---------------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Axial Demand (kips) | 200 | 208 | 210 | 213 |
| Δ_{xx} | 22.7 | 22.7 | 22.7 | 22.7 |
| Δ_{xy} | 2.7 | 2.7 | 2.7 | 2.7 |
| Δ_{yx} | 0 | 0 | 0 | 0 |
| Δ_{yy} | 23.3 | 23.3 | 23.3 | 23.3 |
| Δ_d | 25.05 | 25.05 | 25.05 | 25.05 |
| V/W | 0.71 | 0.75 | 0.81 | 0.86 |
| $4*\Delta_d/H$ | 0.08 | 0.08 | 0.09 | 0.09 |
| P-Delta effect required? | NO | NO | NO | NO |

Assumes gross composite section properties for the pile

From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 for MCE event; MOTEMS 3104F.4.2
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3

P-Delta Check [MOTEMS 3104F.4.3] - Corroded Pile Section Properties

| | Sea-side Pile | | | Land-side Pile |
|--------------------------|---------------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 |
| Axial Demand (kips) | 200 | 208 | 210 | 213 |
| Δ_{xx} | 44.1 | 44.1 | 44.1 | 44.1 |
| Δ_{xy} | 6.6 | 6.6 | 6.6 | 6.6 |
| Δ_{yx} | 0 | 0 | 0 | 0 |
| Δ_{yy} | 45.3 | 45.3 | 45.3 | 45.3 |
| Δ_d | 49.10 | 49.10 | 49.10 | 49.10 |
| V/W | 0.57 | 0.61 | 0.66 | 0.72 |
| $4*\Delta_d/H$ | 0.16 | 0.17 | 0.17 | 0.18 |
| P-Delta effect required? | NO | NO | NO | NO |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 for MCE event; MOTEMS 3104F.4.2
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Wharf

Designed By: V. Phan
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Pile Displacement Capacity - Gross Pile Section Properties

| | Demand | Capacity | Check | D/C Ratio |
|-----|--------|----------|-------|-----------|
| OLE | 4 | 19 | GOOD | 0.21 |
| CLE | 12 | 31 | GOOD | 0.39 |
| MCE | 25 | 42 | GOOD | 0.60 |

Assumes gross composite section properties for the pile

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

Pile Displacement Capacity - Corroded Pile Section Properties

| | Demand | Capacity | Check | D/C Ratio |
|-----|--------|----------|-------|-----------|
| OLE | 7 | 33 | GOOD | 0.21 |
| CLE | 20 | 45 | GOOD | 0.44 |
| MCE | 49 | 56 | GOOD | 0.88 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Option 5H - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Container Wharf

Designed By: G. Jankhah
Date: February 4, 2013
Checked By: H. Guan
Date: February 11, 2013

Background: The following summarizes the displacement demand and capacity of the wharf system under seismic demands. The demand to capacity ratio of the structure is verified to be equal to or less than 1.0 for the MCE, CLE, and OLE seismic events.

Design Assumptions: The displacement demand was determined from a linear response spectrum analysis performed on SAP2000 using a 3D model. For the foundation, a point of fixity for each pile was approximated based on LPILE analysis.

The displacement capacity was determined from nonlinear static pushover analysis performed on SAP2000 using 2D models of a critical cross sections. Pile concrete and steel strains were limited based on MOTEMS and POLB seismic criteria and is summarized in Appendix B - Structures Design Criteria. Expected material properties were used for this analysis.

References: (1) Marine Oil Terminal Engineering and Maintenance Standards (MOTEMS), California Building Code, Section 3104F, 2011
 (2) Port of Long Beach (POLB) Wharf Design Criteria, Section 4, v 3.0, 2012
 (3) AASHTO LRFD Bridge Design Specifications, Section 10, 6th Edition with Interims, 2012

RC Pile Properties

| | | |
|-------------------------------------|-------|-----|
| Expected Rebar Steel Yield Strength | 66 | ksi |
| Pile Rebar Size | 11 | |
| Pile Rebar Diameter | 1.375 | in |

MOTEMS 3107F.2.1.1
 Analysis is based on 24 #11 bars in connection to pile cap

Steel Pile Properties

| | | |
|-------------------------------|-------|-----------------|
| Expected Steel Yield Strength | 55 | ksi |
| Modulus of Elasticity | 29000 | ksi |
| Pile Diameter | 48 | in |
| Pile Wall Thickness | 0.75 | in |
| Pile Area | 111 | in ² |
| Pile Moment of Inertia | 31077 | in ⁴ |
| Plastic Modulus | 1675 | in ³ |

MOTEMS 3107F.2.1.1
 Per conceptual plans
 Per conceptual plans; Gross Thickness

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 5H - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Container Wharf

Designed By: G. Jankhah
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Point of Fixity Computation - Corroded Pile Section Properties

| | Sea-side Pile | | | | | | Land-side Pile |
|--|---------------|--------|--------|--------|--------|--------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| Pile Spacing (ft) | 12 | 18 | 18 | 18 | 18 | 16 | 16 |
| Axial Demand (kips) | 260 | 216 | 205 | 209 | 203 | 171 | 252 |
| P-Multiplier | 0.30 | 0.60 | 0.60 | 0.60 | 0.60 | 0.50 | 0.50 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -49.30 | -46.90 | -43.30 | -39.70 | -36.10 | -32.50 | -29.30 |
| Top of Clay Layer | -55.00 | -55.00 | -55.00 | -55.00 | -55.00 | -55.00 | -55.00 |
| Approx. No. Diameters to Fixity ¹ | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Approx. Point of Fixity Elevation | -71.3 | -68.9 | -65.3 | -61.7 | -58.1 | -54.5 | -51.3 |
| SAP Mudline Shear (kips) | 97.0 | 104.0 | 117.0 | 130.0 | 147.0 | 166.0 | 172.0 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

P-Delta Check [MOTEMS 3104F.4.3] - Corroded Pile Section Properties

| | Sea-side Pile | | | | | | Land-side Pile |
|--------------------------|---------------|-------|-------|-------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| Axial Demand (kips) | 176 | 204 | 209 | 209 | 207 | 157 | 169 |
| Δ_{xx} | 38 | 35 | 32 | 30.5 | 30 | 30 | 30 |
| Δ_{xy} | 30 | 30 | 30 | 30 | 30 | 30 | 30 |
| Δ_{yx} | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Δ_{yy} | 38 | 38 | 38 | 38 | 38 | 38 | 38 |
| Δ_d | 39.10 | 39.10 | 39.10 | 39.10 | 39.10 | 39.10 | 39.10 |
| V/W | 0.55 | 0.51 | 0.56 | 0.62 | 0.71 | 1.06 | 1.02 |
| $4*\Delta_d/H$ | 0.12 | 0.13 | 0.13 | 0.14 | 0.14 | 0.15 | 0.15 |
| P-Delta effect required? | NO | NO | NO | NO | NO | NO | NO |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

From SAP Output

From SAP Output

From SAP Output

From SAP Output

From SAP Output

for MCE event; MOTEMS 3104F.4.2

MOTEMS 3104F.4.3

MOTEMS 3104F.4.3

MOTEMS 3104F.4.3

Pile Displacement Capacity - Corroded Pile Section Properties

| | Demand | Capacity | Check | D/C |
|-----|--------|----------|-------|------|
| OLE | N/A | N/A | N/A | N/A |
| CLE | 16 | 39 | GOOD | 0.41 |
| MCE | 39 | 51 | GOOD | 0.76 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Top Hinge Failed

Top Hinge Failed

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design
Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Trestle

Designed By: G. Jankhah
Date: February 4, 2013
Checked By: H. Guan
Date: February 11, 2013

Background: The following summarizes the displacement demand and capacity of the wharf system under seismic demands. The demand to capacity ratio of the structure is verified to be equal to or less than 1.0 for the MCE, CLE, and OLE seismic events.

Design Assumptions: The displacement demand was determined from a linear response spectrum analysis performed on SAP2000 using a 3D model. For the foundation, a point of fixity for each pile was approximated based on LPILE analysis.

The displacement capacity was determined from nonlinear static pushover analysis performed on SAP2000 using 2D models of a critical cross sections. Pile concrete and steel strains were limited based on MOTEMS and POLB seismic criteria and is summarized in Appendix B - Structures Design Criteria. Expected material properties were used for this analysis.

- References:** (1) Marine Oil Terminal Engineering and Maintenance Standards (MOTEMS), California Building Code, Section 3104F, 2011
(2) Port of Long Beach (POLB) Wharf Design Criteria, Section 4, v 3.0, 2012
(3) AASHTO LRFD Bridge Design Specifications, Section 10, 6th Edition with Interims, 2012

RC Pile Properties

| | | | |
|-------------------------------------|-------|-----|--|
| Expected Rebar Steel Yield Strength | 66 | ksi | MOTEMS 3107F.2.1.1 |
| Pile Rebar Size | 11 | | Analysis is based on 24 #11 bars in connection to pile cap |
| Pile Rebar Diameter | 1.375 | in | |

Steel Pile Properties

| | | | |
|-------------------------------|-------|-----------------|---------------------------------------|
| Expected Steel Yield Strength | 55 | ksi | MOTEMS 3107F.2.1.1 |
| Modulus of Elasticity | 29000 | ksi | |
| Pile Diameter | 48 | in | Per conceptual plans |
| Pile Wall Thickness | 0.75 | in | Per conceptual plans; Gross Thickness |
| Pile Area | 111 | in ² | |
| Pile Moment of Inertia | 31077 | in ⁴ | |
| Plastic Modulus | 1675 | in ³ | |

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Trestle

Designed By: G. Jankhah
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Point of Fixity Computation - Gross Pile Section Properties

| | Sea-side Pile | | | | | | | | | Land-side Pile |
|-----------------------------------|---------------|-------|-------|-------|-------|-------|-------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| Pile Spacing (ft) | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 14 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 181 | 124 |
| P-Multiplier | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.40 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 | 10.6 | 14.6 | 17.4 |
| Top of Clay Layer | -17.4 | -15 | -15 | -15 | -15 | -15 | -15 | -15 | -15 | -15 |
| Approx. No. Diameters to Fixity | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Approx. Point of Fixity Elevation | -29.4 | -25.4 | -21.4 | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 5.4 |
| SAP Mudline Shear (kips) | 177.0 | 177.0 | 179.0 | 178.0 | 173.0 | 160.0 | 139.0 | 111.0 | 122.0 | 206.0 |

Assumes gross composite section properties for pile

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

Point of Fixity Computation - Corroded Pile Section Properties

| | Sea-side Pile | | | | | | | | | Land-side Pile |
|-----------------------------------|---------------|-------|-------|-------|-------|-------|-------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 15 |
| Pile Spacing (ft) | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 14 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 181 | 125 |
| P-Multiplier | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.40 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 2.6 | 6.6 | 10.6 | 14.6 | 17.4 |
| Top of Clay Layer | -17.4 | -15 | -15 | -15 | -15 | -15 | -15 | -15 | -15 | -15 |
| Approx. No. Diameters to Fixity | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Approx. Point of Fixity Elevation | -33.4 | -29.4 | -25.4 | -21.4 | -17.4 | -13.4 | -9.4 | -5.4 | -1.4 | 1.4 |
| SAP Mudline Shear (kips) | 172.0 | 177.0 | 185.0 | 189.0 | 189.0 | 180.0 | 161.0 | 135.0 | 159.0 | 270.0 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3 and higher

Approximate

Determined through LPILE analysis

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design
Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Trestle

Designed By: G. Jankhah
Date: February 4, 2013
Checked By: H. Guan
Date: February 11, 2013

P-Delta Check [MOTEMS 3104F.4.3] - Gross Pile Section Properties

Assumes gross composite section properties for the pile

| | Sea-side Pile | | | | | | | | | Land-side Pile |
|--------------------------|---------------|------|------|------|------|------|------|------|------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 15 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 181 | 124 |
| Δ_{xx} | 8.5 | 7.3 | 6.2 | 5 | 4 | 3 | 2 | 1.2 | 1 | 1.3 |
| Δ_{xy} | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 |
| Δ_{yx} | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Δ_{yy} | 2.4 | 2.4 | 2.4 | 2.4 | 2.4 | 2.4 | 2.4 | 2.4 | 2.4 | 2.4 |
| Δ_d | 8.59 | 7.40 | 6.32 | 5.15 | 4.18 | 3.24 | 2.62 | 2.58 | 2.57 | 2.58 |
| V/W | 0.88 | 0.88 | 0.86 | 0.84 | 0.91 | 0.76 | 0.66 | 0.53 | 0.67 | 1.66 |
| $4*\Delta_d/H$ | 0.05 | 0.04 | 0.04 | 0.03 | 0.03 | 0.03 | 0.02 | 0.02 | 0.03 | 0.03 |
| P-Delta effect required? | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |

From SAP Output
From SAP Output
From SAP Output
From SAP Output
From SAP Output
for MCE event; MOTEMS 3104F.4.2
MOTEMS 3104F.4.3
MOTEMS 3104F.4.3
MOTEMS 3104F.4.3

P-Delta Check [MOTEMS 3104F.4.3] - Corroded Pile Section Properties

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

| | Sea-side Pile | | | | | | | | | Land-side Pile |
|--------------------------|---------------|-------|-------|-------|-------|------|------|------|------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 15 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 181 | 125 |
| Δ_{xx} | 20.6 | 17.9 | 15.2 | 12.5 | 9.9 | 7.4 | 5 | 3.1 | 2.7 | 3.6 |
| Δ_{xy} | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 |
| Δ_{yx} | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Δ_{yy} | 6.2 | 6.2 | 6.2 | 6.2 | 6.2 | 6.2 | 6.2 | 6.2 | 6.2 | 6.2 |
| Δ_d | 20.86 | 18.19 | 15.55 | 12.92 | 10.42 | 8.09 | 6.79 | 6.69 | 6.67 | 6.71 |
| V/W | 0.85 | 0.88 | 0.89 | 0.90 | 0.99 | 0.85 | 0.76 | 0.64 | 0.88 | 2.16 |
| $4*\Delta_d/H$ | 0.10 | 0.10 | 0.09 | 0.08 | 0.07 | 0.06 | 0.05 | 0.06 | 0.06 | 0.07 |
| P-Delta effect required? | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |

From SAP Output
From SAP Output
From SAP Output
From SAP Output
From SAP Output
for MCE event; MOTEMS 3104F.4.2
MOTEMS 3104F.4.3
MOTEMS 3104F.4.3
MOTEMS 3104F.4.3

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design
 Option 1 - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section A - Trestle

Designed By: G. Jankhah
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Pile Displacement Capacity - Gross Pile Section Properties

| | Y | | | | X | | | |
|-----|--------|----------|-------|-----|--------|----------|-------|-----|
| | Demand | Capacity | Check | D/C | Demand | Capacity | Check | D/C |
| OLE | 1.9 | N/A | N/A | N/A | 3.7 | N/A | N/A | N/A |
| CLE | 3.3 | N/A | N/A | N/A | 7.3 | N/A | N/A | N/A |
| MCE | 3.6 | 13.9 | GOOD | 0.3 | 8.6 | 23 | GOOD | 0.4 |

Assumes gross composite section properties for pile

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

Pile Displacement Capacity - Corroded Pile Section Properties

| | Y | | | | X | | | |
|-----|--------|----------|-------|-----|--------|----------|-------|-----|
| | Demand | Capacity | Check | D/C | Demand | Capacity | Check | D/C |
| OLE | 3.2 | N/A | N/A | N/A | 5.7 | N/A | N/A | N/A |
| CLE | 6.9 | N/A | N/A | N/A | 13.6 | N/A | N/A | N/A |
| MCE | 9.1 | 17.5 | GOOD | 0.5 | 20.9 | 26.4 | GOOD | 0.8 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

US Army Corps of Engineers
Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
Option 5H - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Trestle

Designed By: G. Jankhah
Date: February 4, 2013
Checked By: H. Guan
Date: February 11, 2013

Background: The following summarizes the displacement demand and capacity of the wharf system under seismic demands. The demand to capacity ratio of the structure is verified to be equal to or less than 1.0 for the MCE, CLE, and OLE seismic events.

Design Assumptions: The displacement demand was determined from a linear response spectrum analysis performed on SAP2000 using a 3D model. For the foundation, a point of fixity for each pile was approximated based on LPILE analysis.

The displacement capacity was determined from nonlinear static pushover analysis performed on SAP2000 using 2D models of a critical cross sections. Pile concrete and steel strains were limited based on MOTEMS and POLB seismic criteria and is summarized in Appendix B - Structures Design Criteria. Expected material properties were used for this analysis.

- References:** (1) Marine Oil Terminal Engineering and Maintenance Standards (MOTEMS), California Building Code, Section 3104F, 2011
 (2) Port of Long Beach (POLB) Wharf Design Criteria, Section 4, v 3.0, 2012
 (3) AASHTO LRFD Bridge Design Specifications, Section 10, 6th Edition with Interims, 2012

RC Pile Properties

| | | |
|-------------------------------------|-------|-----|
| Expected Rebar Steel Yield Strength | 66 | ksi |
| Pile Rebar Size | 11 | |
| Pile Rebar Diameter | 1.375 | in |

MOTEMS 3107F.2.1.1
 Analysis is based on 24 #11 bars in connection to pile cap

Steel Pile Properties

| | | |
|-------------------------------|-------|-----------------|
| Expected Steel Yield Strength | 55 | ksi |
| Modulus of Elasticity | 29000 | ksi |
| Pile Diameter | 48 | in |
| Pile Wall Thickness | 0.75 | in |
| Pile Area | 111 | in ² |
| Pile Moment of Inertia | 31077 | in ⁴ |
| Plastic Modulus | 1675 | in ³ |

MOTEMS 3107F.2.1.1
 Per conceptual plans
 Per conceptual plans; Gross Thickness

Point of Fixity Computation - Gross Pile Section Properties

Assumes gross composite section properties

| | Sea-side Pile | | | | | | | | | | | | | | Land-side Pile |
|-----------------------------------|---------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 |
| Pile Spacing (ft) | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 208 | 180 |
| P-Multiplier (Y) | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 |
| P-Multiplier (X) | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -26.2 | -22.2 | -18.2 | -14.2 | -10.2 | -6.2 | -2.2 | 1.8 | 5.8 | 9.8 | 13.8 | 17.8 | 21.8 | 25.8 | 29.8 |
| Top of Clay Layer | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 |
| Approx. No. Diameters to Fixity | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Approx. Point of Fixity Elevation | -38.2 | -34.2 | -30.2 | -26.2 | -22.2 | -18.2 | -14.2 | -10.2 | -6.2 | -2.2 | 1.8 | 5.8 | 9.8 | 13.8 | 17.8 |
| SAP Mudline Shear (kips) X | 166.0 | 169.0 | 174.0 | 178.0 | 180.0 | 182.0 | 182.0 | 180.0 | 175.0 | 167.0 | 152.0 | 136.0 | 134.0 | 209.0 | 290.0 |

Approximate; Based on SAP Output
 Based on AASHTO Table 10-7.2.4-1 for Row 3+
 Approximate
 Determined through LPILE analysis

Point of Fixity Computation - Corroded Pile Section Properties

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

| | Sea-side Pile | | | | | | | | | | | | | | Land-side Pile |
|-----------------------------------|---------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 |
| Pile Spacing (ft) | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 208 | 180 |
| P-Multiplier | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 | 0.70 |
| Top of Pile Elevation | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 | 33.00 |
| Mudline Elevation | -26.2 | -22.2 | -18.2 | -14.2 | -10.2 | -6.2 | -2.2 | 1.8 | 5.8 | 9.8 | 13.8 | 17.8 | 21.8 | 25.8 | 29.8 |
| Top of Clay Layer | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 | -55 |
| Approx. No. Diameters to Fixity | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Approx. Point of Fixity Elevation | -42.2 | -38.2 | -34.2 | -30.2 | -26.2 | -22.2 | -18.2 | -14.2 | -10.2 | -6.2 | -2.2 | 1.8 | 5.8 | 9.8 | 13.8 |
| SAP Mudline Shear (kips) | 134.0 | 141.0 | 151.0 | 161.0 | 172.0 | 183.0 | 192.0 | 201.0 | 208.0 | 209.0 | 176.0 | 143.0 | 143.0 | 236.0 | 406.0 |

Approximate; Based on SAP Output
 AASHTO Table 10-7.2.4-1 for Row 3 and higher
 Approximate
 Determined through LPILE analysis

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 5H - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Trestle

Designed By: G. Jankhah
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Assumes gross composite section properties

P-Delta Check [MOTEMS 3104F.4.3] - Gross Pile Section Properties

| | Sea-side Pile | | | | | | | | | | | | | | Land-side Pile |
|--------------------------|---------------|------|------|------|------|------|------|------|------|------|------|------|------|------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 208 | 180 |
| Δ_{xx} | 10.7 | 9.6 | 8.5 | 7.3 | 6.3 | 5.3 | 4.3 | 3.4 | 2.6 | 1.9 | 1.3 | 0.8 | 0.5 | 0.5 | 0.8 |
| Δ_{xy} | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 | 0.5 |
| Δ_{yx} | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Δ_{yy} | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 |
| Δ_d | 10.74 | 9.64 | 8.55 | 7.36 | 6.37 | 5.38 | 4.40 | 3.52 | 2.76 | 2.11 | 1.60 | 1.57 | 1.56 | 1.56 | 1.57 |
| V/W | 0.82 | 0.84 | 0.83 | 0.84 | 0.94 | 0.86 | 0.86 | 0.85 | 0.83 | 0.79 | 0.72 | 0.64 | 0.64 | 1.00 | 1.61 |
| $4*\Delta_d/H$ | 0.05 | 0.05 | 0.05 | 0.04 | 0.04 | 0.04 | 0.03 | 0.03 | 0.02 | 0.02 | 0.02 | 0.02 | 0.02 | 0.03 | 0.03 |
| P-Delta effect required? | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |

From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 for MCE event; MOTEMS 3104F.4.2
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3

P-Delta Check [MOTEMS 3104F.4.3] - Corroded Pile Section Properties

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

| | Sea-side Pile | | | | | | | | | | | | | | Land-side Pile |
|--------------------------|---------------|-------|-------|-------|-------|-------|-------|------|------|------|------|------|------|------|----------------|
| Row | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 |
| Axial Demand (kips) | 202 | 202 | 209 | 211 | 191 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 211 | 208 | 180 |
| Δ_{xx} | 23.6 | 21.4 | 19.2 | 16.9 | 14.7 | 12.6 | 10.5 | 8.6 | 6.8 | 5.1 | 3.6 | 2.3 | 1.3 | 1.4 | 2.4 |
| Δ_{xy} | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Δ_{yx} | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Δ_{yy} | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 |
| Δ_d | 23.70 | 21.51 | 19.32 | 17.03 | 14.85 | 12.78 | 10.72 | 8.86 | 7.13 | 5.53 | 4.24 | 4.16 | 4.12 | 4.12 | 4.16 |
| V/W | 0.66 | 0.70 | 0.72 | 0.76 | 0.90 | 0.87 | 0.91 | 0.95 | 0.99 | 0.99 | 0.83 | 0.68 | 0.68 | 1.13 | 2.26 |
| $4*\Delta_d/H$ | 0.11 | 0.10 | 0.10 | 0.09 | 0.08 | 0.08 | 0.07 | 0.06 | 0.06 | 0.05 | 0.04 | 0.04 | 0.05 | 0.06 | 0.07 |
| P-Delta effect required? | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO | NO |

From SAP Output
 From SAP Output
 From SAP Output
 From SAP Output
 for MCE event; MOTEMS 3104F.4.2
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3
 MOTEMS 3104F.4.3

US Army Corps of Engineers
 Port of Anchorage - Intermodal Expansion Project - 15% Design - Preliminary Calculations
 Option 5H - 48" Steel Hybrid Pipe Pile Lateral Seismic Design - Section C - Trestle

Designed By: G. Jankhah
 Date: February 4, 2013
 Checked By: H. Guan
 Date: February 11, 2013

Pile Displacement Capacity - Gross Pile Section Properties

| | Y | | | | X | | | |
|-----|--------|----------|-------|-----|--------|----------|-------|-----|
| | Demand | Capacity | Check | D/C | Demand | Capacity | Check | D/C |
| OLE | 1.6 | 4 | GOOD | 0.4 | 4.1 | 14.8 | GOOD | 0.3 |
| CLE | 3.3 | 7.5 | GOOD | 0.4 | 9.3 | 20 | GOOD | 0.5 |
| MCE | 3.6 | 9.8 | GOOD | 0.4 | 10.7 | 25 | GOOD | 0.4 |

Assumes gross composition section properties for the pile

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed

Pile Displacement Capacity - Corroded Pile Section Properties

| | Y | | | | X | | | |
|-----|--------|----------|-------|-----|--------|----------|-------|-----|
| | Demand | Capacity | Check | D/C | Demand | Capacity | Check | D/C |
| OLE | 2.8 | 4.5 | GOOD | 0.6 | 6.3 | 13.5 | GOOD | 0.5 |
| CLE | 5.6 | 9.6 | GOOD | 0.6 | 14.4 | 25 | GOOD | 0.6 |
| MCE | 8.2 | 11.8 | GOOD | 0.7 | 23.7 | 31 | GOOD | 0.8 |

Assumes steel section above mudline is fully corroded and 0.3" loss of steel below mudline

Top Hinge Failed
 Top Hinge Failed
 Top Hinge Failed